

**UNIVERSITY OF BOLTON**  
**SCHOOL OF BUILT ENVIRONMENT &  
ENGINEERING – RAK CAMPUS**  
**BSc(HONS) CIVIL ENGINEERING**  
**SEMESTER THREE EXAMINATION 2010/2011**  
**GEOTECHNICS**  
**MODULE NO: BLT3012**

Date: Tuesday 30 August 2011

Time: 1.00 pm – 3.00 pm

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**INSTRUCTIONS TO CANDIDATES:**

There are **FOUR** questions.

Answer **THREE** questions.

All questions carry equal marks.

Marks for parts of questions are shown in brackets.

Supplementary Geotechnical Information is provided on page 9.

Lined Graph Paper and Supplementary Answer Sheets are available for your use.

Detach Table Q4b and insert into your Answer Booklet using Treasury Tags.

Ensure that you write your Candidate Number or Desk Number on each Figure, Supplementary Sheet or Sheet of Graph Paper you use to answer the selected questions.

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School of the Built Environment & Engineering – RAK Campus  
 BSc(Hons) Civil Engineering  
 Semester Three Examination 2010/2011  
 Geotechnics  
 Module No. BLT3012

1. A reinforced concrete gravity wall, 1.5m wide at its top and 2m wide at its base, is resting upon and also retains 5.0m of granular soil. A cross section through the wall is shown in Figure Q1.

Other relevant parameters are:

<b><u>Granular Soil</u></b>	Bulk Unit weight	18.0 kN/m <sup>3</sup>
	Saturated Unit Weight	19.5 kN/m <sup>3</sup>
	Effective friction angle $\phi'$	28°
	Ultimate bearing capacity of base	200 kN/m <sup>2</sup>

**Reinforced concrete**                      Unit weight 24 kN/m<sup>3</sup>

**Groundwater**                              2.0m below upper retained surface  
 Unit Weight of Water    10kN/m<sup>3</sup>

**Surcharge**                                    15kN/m<sup>2</sup> on upper retained surface

**Formulae**     $K_a = (1 - \sin \phi) / (1 + \sin \phi)$      $K_p = (1 + \sin \phi) / (1 - \sin \phi)$

- a) Determine, by calculation, the resultant thrust acting on the back of the retaining wall. (14 marks)
- b) Determine the height of the resultant thrust above the base of the retaining wall and hence determine the Factor of Safety against overturning (ignore water pressures on the base of the foundation). (4 marks)
- c) What are the advantages and disadvantages of using an embedded retaining wall instead of a gravity retaining wall? Ensure that your answer discusses construction practicality as well as technical aspects. (7 marks)

**Total 25 marks**

**Question 1 continued over the page...**

School of the Built Environment & Engineering – RAK Campus  
BSc(Hons) Civil Engineering  
Semester Three Examination 2010/2011  
Geotechnics  
Module No. BLT3012

**Question 1 continued**

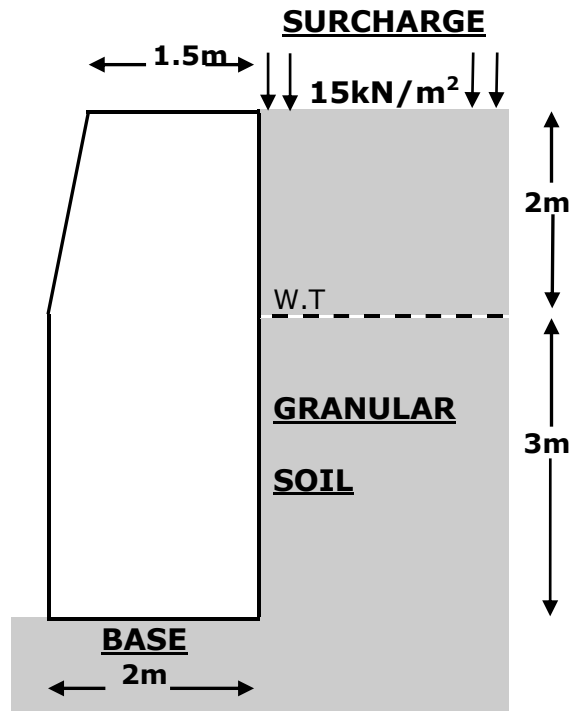


Figure Q1

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School of the Built Environment & Engineering – RAK Campus  
 BSc(Hons) Civil Engineering  
 Semester Three Examination 2010/2011  
 Geotechnics  
 Module No. BLT3012

2. a) Publication of the final National Annex of BS EN 1997-1:2004 and BS EN 1997-2:2007 has made an impact on design. Summarise the key changes and specifically describe how foundation design in the United Kingdom has been, and is likely to be affected still further by their introduction.

Ensure that your answer briefly discusses any changes to the way in which the calculations will provide a safe foundation design and refer to the current guidance documents and codes of practice that will be replaced.

(7 marks)

- b) A pad foundation, 2.5m square is to be located at a depth of 1.2m in a uniform bearing stratum of firm clay. The water table level is at an assumed depth of 1.2m below ground level (from a recent and reliable site investigation). The clay soil properties are as follows;

Bulk unit weight	$\gamma = 20.0 \text{ kN/m}^3$
Saturated unit weight	$\gamma_{\text{sat}} = 21.5 \text{ kN/m}^3$
Total Stress Shear Strength Parameters	$c_u = 70.0 \text{ kN/m}^2$
	$\Phi_u = 0^\circ$
Effective Stress Shear Strength Parameters	$c^i = 12.0 \text{ kN/m}^2$
	$\Phi^i = 28^\circ$

Using Table Q2 determine the safe bearing load that the foundation can support in the short term (in kN).

(10 marks)

NOTE: Clearly state any assumptions made in your calculations to determine the safe bearing load in Q2 b). Use the formulae provided on page 9 and also Table Q2 (as appropriate).

- c) Using the data provided for Q2 b) determine the safe bearing load that the foundation can support in the long term (in kN).

(8 marks)

**Total 25 marks**

**Question 2 continued over the page...**

School of the Built Environment & Engineering – RAK Campus  
 BSc(Hons) Civil Engineering  
 Semester Three Examination 2010/2011  
 Geotechnics  
 Module No. BLT3012

**Question 2 continued**

$\phi$	$N_c$	$N_q$	$N_\gamma$
0	5.14	1.0	0
1	5.4	1.1	0
2	5.6	1.2	0
3	5.9	1.3	0
4	6.2	1.4	0
5	6.5	1.6	0.1
6	6.8	1.7	0.1
7	7.2	1.9	0.2
8	7.5	2.1	0.2
9	7.9	2.3	0.3
10	8.4	2.5	0.4
11	8.8	2.7	0.5
12	9.3	3.0	0.6
13	9.8	3.3	0.8
14	10.4	3.6	1.0
15	11.0	3.9	1.2
16	11.6	4.3	1.4
17	12.3	4.8	1.7
18	13.1	5.3	2.1
19	13.9	5.8	2.5
20	14.8	6.4	3.0
21	15.8	7.1	3.5
22	16.9	7.8	4.1
23	18.1	8.7	4.9
24	19.3	9.6	5.7
25	20.7	10.7	6.8
26	22.3	11.9	7.9
27	23.9	13.2	9.3
28	25.8	14.7	10.9
29	27.9	16.4	12.8
30	30.1	18.4	15.1
31	32.7	20.6	17.7
32	35.5	23.2	20.8
33	38.6	26.1	24.4
34	42.2	29.4	28.8
35	46.1	33.3	33.9
36	50.6	37.8	40.0
37	55.6	42.9	47.4
38	61.4	48.9	56.2
39	67.9	56.0	66.8
40	75.3	64.2	79.5

**Table Q2**

Please turn the page

School of the Built Environment & Engineering – RAK Campus  
 BSc(Hons) Civil Engineering  
 Semester Three Examination 2010/2011  
 Geotechnics  
 Module No. BLT3012

3. a) A bored pile (16m long and 450mm in diameter) is to be installed into the following soil profile;

Depth m	Description	Unit Weight kN/m <sup>3</sup>	$c_u$ kN/m <sup>2</sup>	Adhesion Factor $\alpha$
0 – 4	Firm CLAY	21.0	50.0	0.6
4 – 10	Stiff CLAY	21.5	85.0	0.45
10 - 20	Stiff to very stiff CLAY	22.0	130.0	0.45

The above clay strata are all taken to be fully saturated.

- i) Determine the safe load carrying capacity of the pile.

NOTE: Clearly state any assumptions made in your calculations

(14 marks)

- ii) What piling options might be considered if a greater load carrying capacity is required but it is not permitted to extend the piles to a greater depth than 16m.

NOTE: Calculations are NOT required for your answer to Q3a)ii).

(5 marks)

- b) Describe the testing methods that would be adopted to confirm the carrying capacity for a driven pile installed into a loose to medium dense, becoming dense to very dense, fine to coarse sand and gravel. Briefly discuss the testing methods that might be routinely used to verify the carrying capacity of the pile.

(6 marks)

**Total 25 marks**

**Please turn the page**

School of the Built Environment & Engineering – RAK Campus  
 BSc(Hons) Civil Engineering  
 Semester Three Examination 2010/2011  
 Geotechnics  
 Module No. BLT3012

4. a) Briefly discuss the allocation of soil properties and use of an appropriate analysis method to address slope stability for the following situations;
- (i) a steep embankment 6m high formed by compacting thin layers of sand and gravel  
 (3 marks)
- (ii) a 5m deep and 10m wide cutting  $34^\circ$  from the horizontal in a firm clay  
 (3 marks)

- b) A cutting, 8.0m in vertical height, has a side slope of  $30^\circ$  from the horizontal and is excavated in a thick stratum of saturated clay with the following properties;

$$\begin{aligned} \text{Saturated unit weight, } \gamma_{\text{sat}} &= 18.81 \text{ kN/m}^3 \\ \text{Unit weight of Water, } \gamma_w &= 9.81 \text{ kN/m}^3 \end{aligned}$$

Strength parameters with respect to effective stress;

$$\begin{aligned} c' &= 14.5 \text{ kN/m}^2 \\ \phi' &= 15^\circ \end{aligned}$$

A trial slip surface of radius,  $R = 15.0\text{m}$  and sector angle,  $\theta = 57^\circ$ , allows the sector to be divided into 6 slices, each 2.075m wide, where slice number 1 is adjacent to the toe.

Complete the table shown in Table Q4b and hence determine the long term factor of safety,  $F$ , of the trial slip surface using the Swedish (Fellenius) analysis which uses the equation below;

$$F = \frac{c' R \theta_{\text{rad}} + \sum (W \cos \alpha - u l) \tan \phi'}{\sum W \sin \alpha}$$

(15 marks)

- c) Describe how implementation of BS EN 1997 will affect slope stability analysis compared with the current method of analysis as shown in Q4(b) above. In particular discuss how the terms in the equation above may vary according to BS EN 1997.  
 (4 marks)

**Total 25 marks**

**Question 4 continued over the page...**

School of the Built Environment & Engineering – RAK Campus  
 BSc(Hons) Civil Engineering  
 Semester Three Examination 2010/2011  
 Geotechnics  
 Module No. BLT3012

**Question 4 continued**

Slice No.	Slice Width b (m)	Slice Height, h (m)	$\alpha$ (°)	Weight W (kN)	$W \sin \alpha$	$W \cos \alpha$	$l / \cos \alpha$ (m)	$u l$	$(W \cos \alpha - ul)$
1	2.075	0.72	-6.03						
2	2.075	2.10	0.00						
3	2.075	3.20	8.51						
4	2.075	3.93	16.20						
5	2.075	4.29	25.02						
6	2.075	4.41	33.57						
				Sum $\Sigma$				Sum $\Sigma$	

**Candidate Number / Seat Number .....**

**Table Q4b**

Complete the missing rows of data in the cells above in Table Q4b and then use the data to complete your answer to Question Q4 b).

**END OF QUESTIONS**

Please turn the page (for one page of Supplementary Geotechnical Information)

**Please turn the page**

School of the Built Environment & Engineering – RAK Campus  
 BSc(Hons) Civil Engineering  
 Semester Three Examination 2010/2011  
 Geotechnics  
 Module No. BLT3012

**Supplementary Geotechnical Information**

$q_f$  = ultimate bearing capacity

$q_n$  = net bearing capacity =  $q_f - \sigma_o = q_f - \gamma D$  (Total stresses)

=  $q_f - \sigma_o^1 = q_f - (\gamma D - \gamma_w h_w)$  (Effective stresses)

$q_s$  = safe bearing capacity =  $\frac{q_n}{F}$  and  $F = \frac{q_f}{q_n} = 3.0$  usually

$q_a$  = allowable bearing capacity =  $\frac{q_n}{F} + \gamma D = \frac{q_f - \gamma D}{F} + \gamma D$

**Shallow Foundations**

c ,  $\phi$  soil

Terzaghi :  $q_f = c N_c s_c + \gamma D N_q s_q + 0.5 B \gamma N_\gamma s_\gamma$

Where  $N_c$  ;  $N_q$  ;  $N_\gamma$  ;  $s_c$  ;  $s_q$  ;  $s_\gamma$  are bearing capacity and shape factors (from tables)

Shape of footing	$s_c$	$s_q$	$s_\gamma$
Strip	1.0	1.0	1.0
Rectangle	$1.0 + (B/L)(N_q/N_c)$	$1.0 + (B/L)\tan\phi'$	$1.0 - (B/L)0.4$
Circle or square	$1.0 + (N_q/N_c)$	$1.0 + \tan\phi'$	0.6

- modified when Water Table present ,  $\gamma_{sub} = \gamma_{sat} - \gamma_w$

c soil ( $\phi = 0$ )

Skempton :  $q_f = cN_c + \gamma D$

$N_c$  from Skempton's graph (for D/B values)

$\phi$  soil (c = 0)

$q_a$  from graph using corrected N values